

CYCLIC COHESIVE MODEL FOR FATIGUE CRACK GROWTH IN CONCRETE

ASMUS SKAR^{1,*}, PETER N. POULSEN², JOHN F. OLESEN²

¹ Section for Geotechnics and Geology, DTU Byg

² Section for Structural Engineering, DTU Byg



ISMEM 2017

- 2nd International Symposium on Multiscale
Experimental Mechanics: Multiscale Fatigue



Concrete subjected fatigue loading

- Concrete highways and bridges
 - app. 5,000,000-50,000,000 passes over critical point during design period
- Port- and industrial pavements
 - app. 250,000-5,000,000 passes over critical point during design period
- Airport pavements
 - app. 10,000-250,000 passes over critical point during design period



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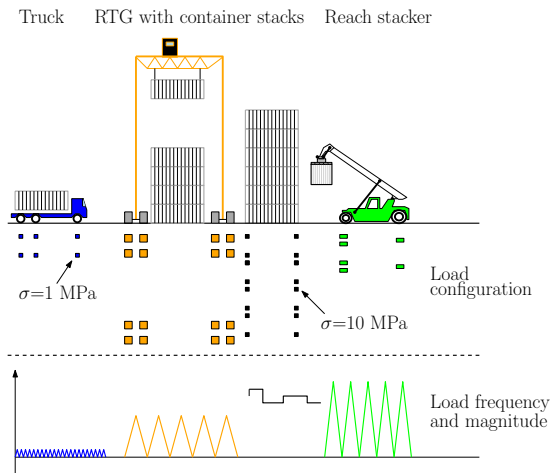
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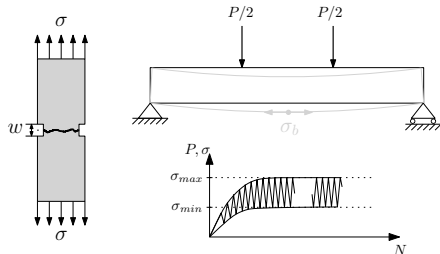
Concrete subjected arbitrary loading

..moreover, some concrete structures are subjected to a wide spectrum of loads and load configurations..



Design methodology

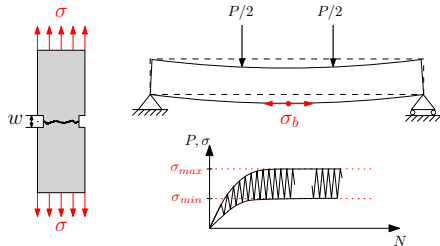
The cyclic behaviour of concrete have mainly been described by Mechanistic-Empirical methods



- Mechanical part: Measured or calculated response (stress or strain level, stress intensity etc.)
- Empirical part: Transfer function (S-N curve) converting the response into number of allowable load cycles
- The M-E models does not distinguish between elastic and inelastic work
- Model parameters A, B and C are regression constants without physical meaning

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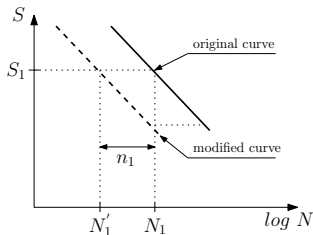


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$$\log N = A + B \frac{\sigma_{max}}{f_t} + C \frac{\sigma_{min}}{f_t}$$

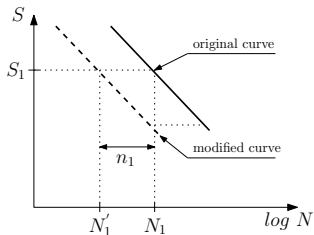


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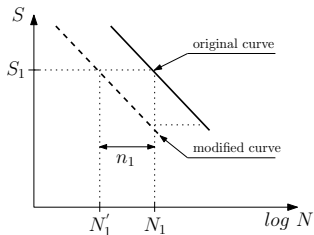


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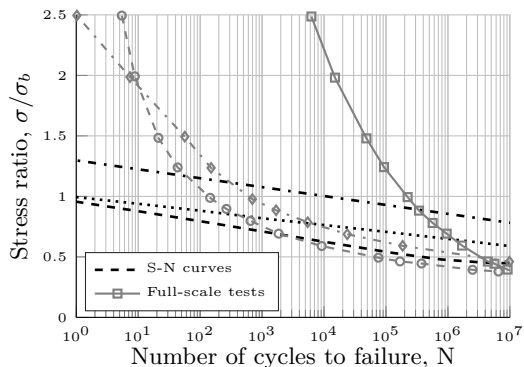
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M-E models vs. full-scale experiments

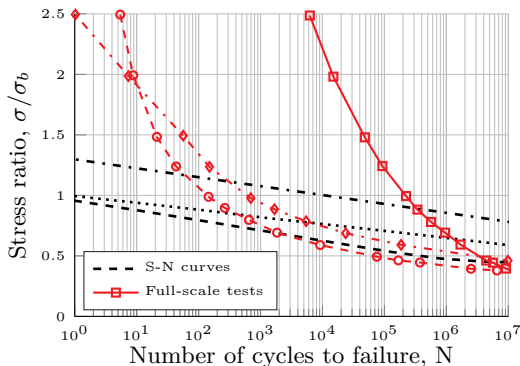
S-N curves and full-scale tests with concrete slabs on ground



- The M-E models have difficulties in accurately predicting realistic behaviour
- Fatigue and monotonic behaviour are badly linked

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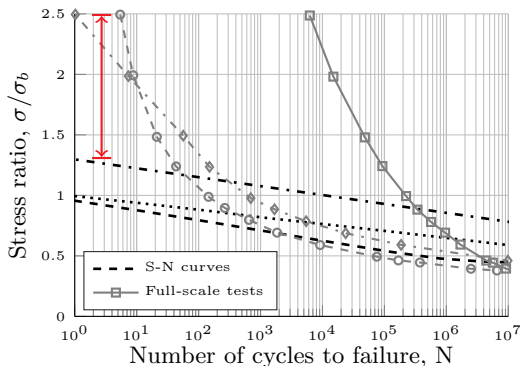
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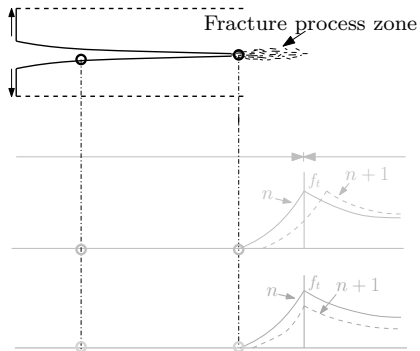
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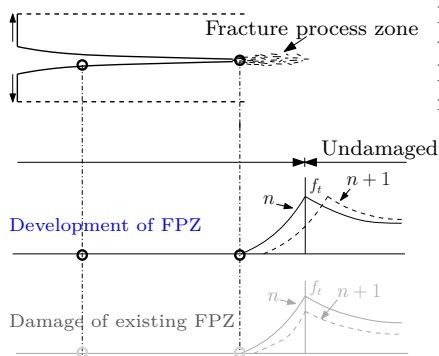
Model framework



In order to create a modelling framework that accounts for the material behaviour during arbitrary loading one need to link:

- The development of the fracture process zone (FPZ);
- the damage of the existing fracture process zone (FPZ), i.e. reduction of the bridging stresses;
- and the monotonic material characteristics

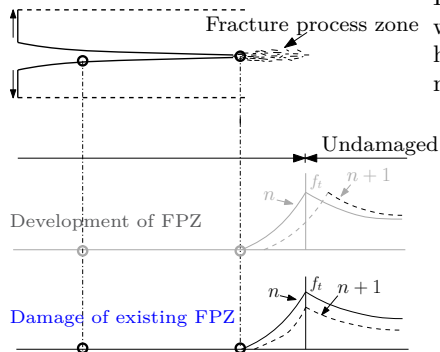
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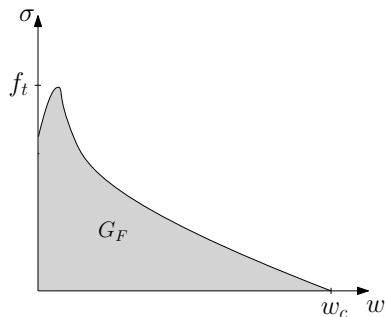
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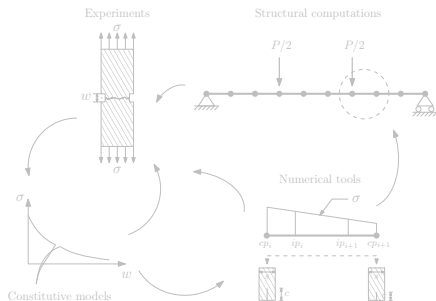
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Multi-scale model

For this purpose the paper presents a multi-scale model. The model consists of three levels:

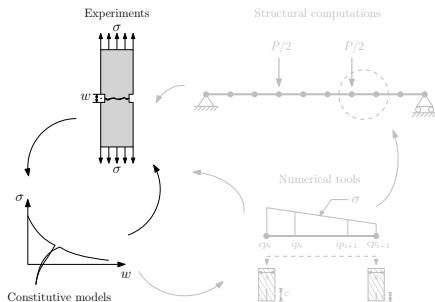
- (i) Fiber level: Develop a cyclic cohesive model considering a fiber of concrete material in uni-axial tension
- (ii) Hinge level: Incorporate the fiber response in a hinge model for description of bending fracture
- (iii) Beam level: For structural analysis, the hinge model is then applied as a constitutive model in a non-linear beam element



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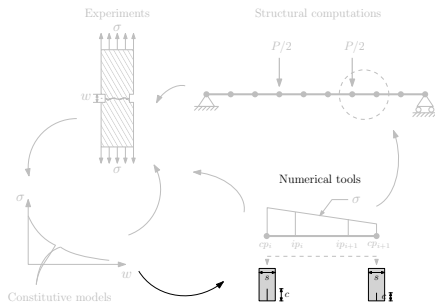
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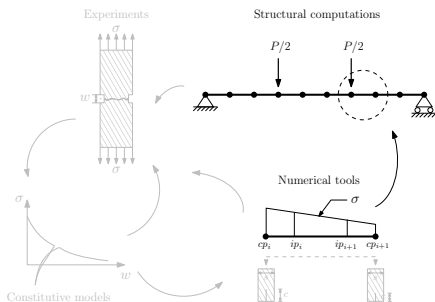
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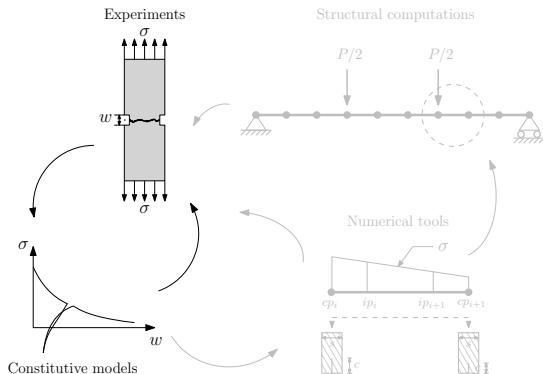
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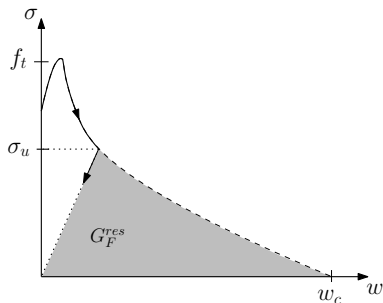


(i) Fiber level: Energy based cyclic cohesive model



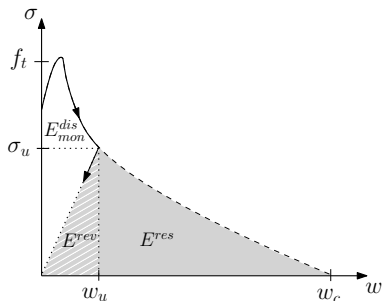
Energy based fatigue model

- **Reduction of bridging stresses is controlled by deterioration of the fracture energy**
- Damage initiation is given by simple relations from geometrical interpretation
- Unloading- and reloading follow straight lines
- Modifications of the model possible, e.g. adjusting the unloading response
- The deterioration of the fracture energy is controlled by the work during fatigue loading, defined as the area below the curve
- A simple linear relationship is selected, where k_{fat} is used to scale the energy dissipated during fatigue loading as a fraction of accumulated work



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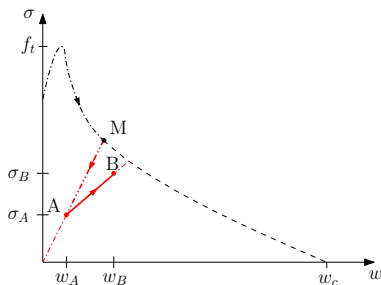
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$$G_{F,i}^{res} = G_F - E_{mon}^{dis} = E^{rev} + E^{res}$$

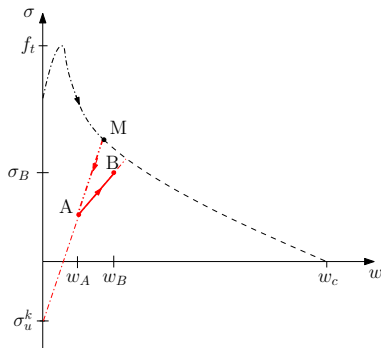
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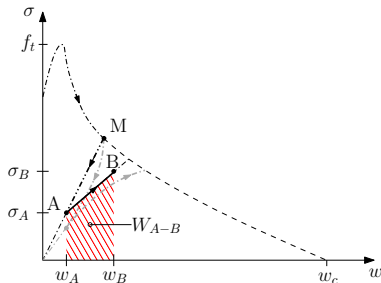
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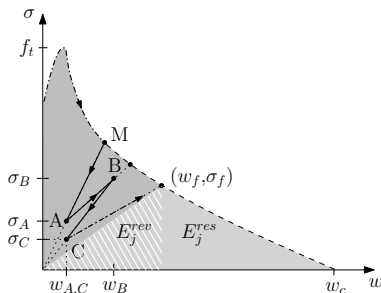
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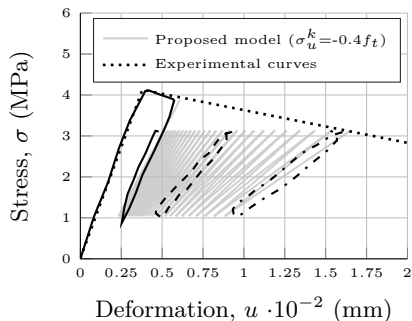
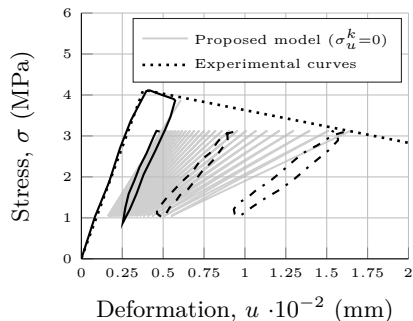
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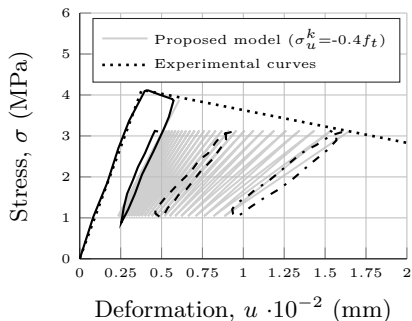
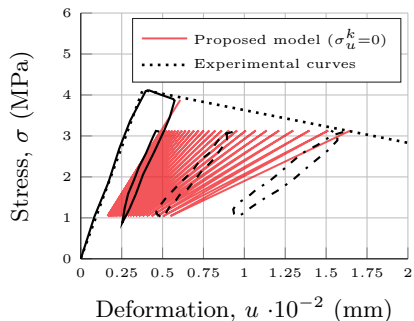
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Numerical study of cracked concrete in uni-axial tension (Plizzari et al., 1997)



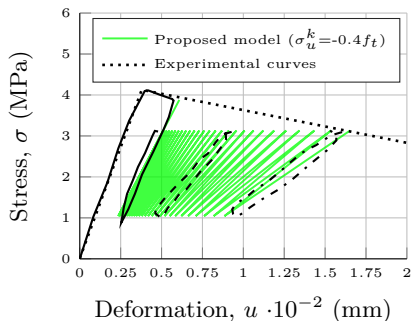
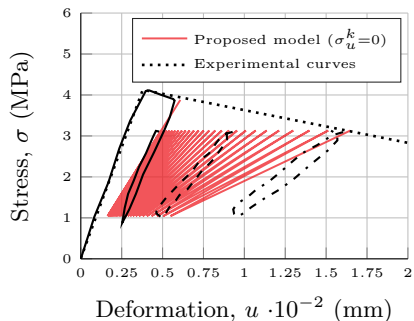
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- Realistic response with unloading towards σ_k^u (suitable for inverse analysis)
- Few data available, thus experimental validation of k_{fat} is difficult

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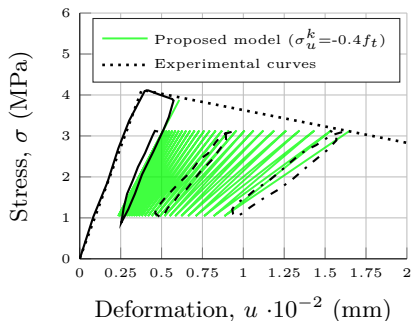
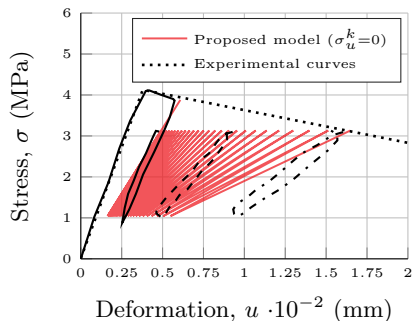
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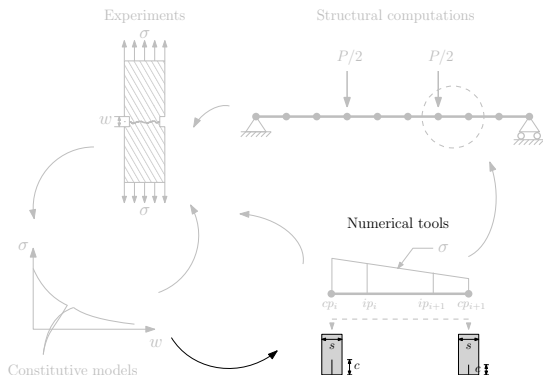
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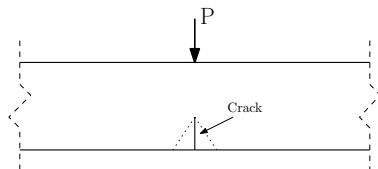
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(ii) Hinge level: Hinge model for description of bending fracture



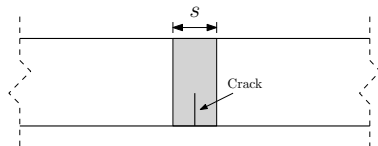
The hinge model - basic assumptions

- **A crack influence the stress and strain field of a structure locally**
- The discontinuity is expected to vanish outside a certain width, defined as the hinge width s
- Thus, the propagation of a crack can be modelled as a hinge, whereas the rest of the beam can be treated as elastic bulk material



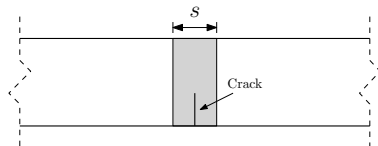
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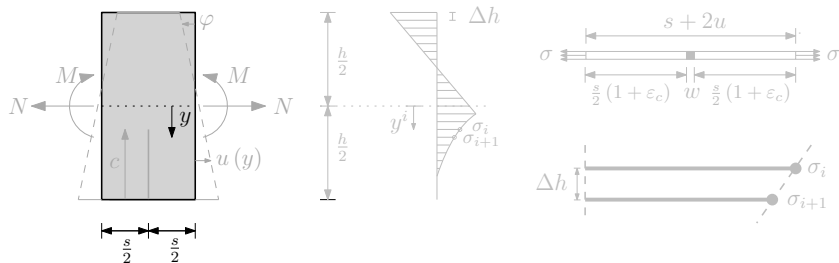


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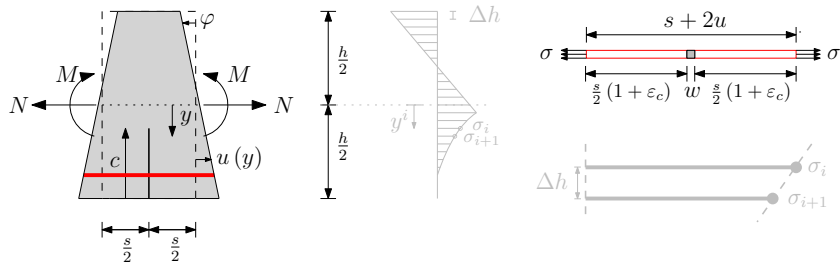
The hinge model - basic mechanics



- From the elongation of a fiber located at, y_i , and the softening response, $\sigma_i = b_i - a_i s$,
- the relevant stresses can be found for each fiber in the cracked phases
- The sectional forces can be found from integration over the hinge height, i.e. the sum of contributions from each strip

$$N(\bar{\varepsilon}_0, \bar{\kappa}) = t \int_{-h/2}^{h/2} \sigma_c dy = \sum_{i=1}^n N_i \quad , \quad M(\bar{\varepsilon}_0, \bar{\kappa}) = t \int_{-h/2}^{h/2} \sigma_c y dy = \sum_{i=1}^n M_i$$

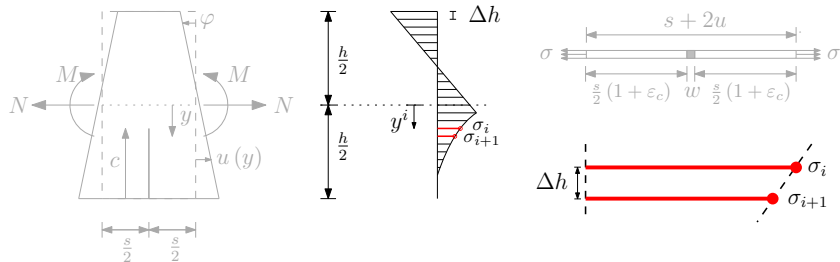
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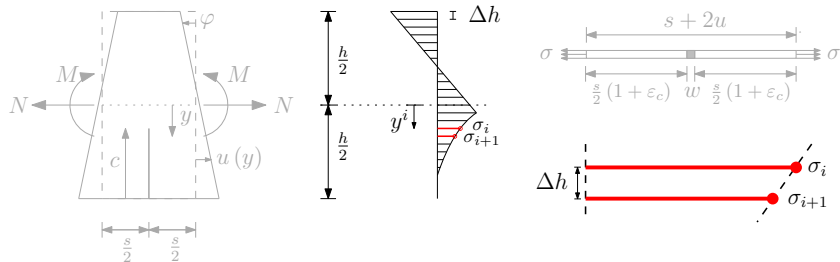
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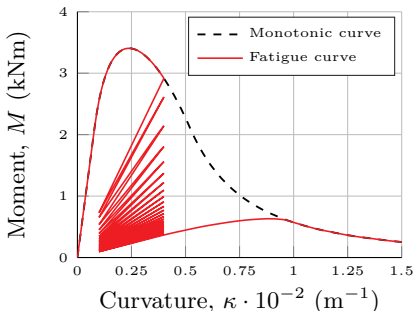


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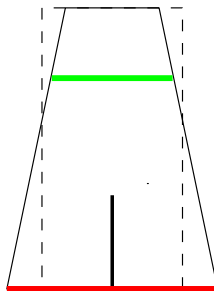
Cyclic response of fiber-hinge

- **Considering a hinge subjected to constant curvature loading**
- Visualising the response of a fiber on the upper part of the hinge (green) and the bottom fiber (red)
- The bottom fiber enters fatigue after the first load step, as the lower part of the hinge deteriorate, new fibers in the upper part are activated
- The upper fiber is first in compression, then in linear elastic tension, before entering a short stage of low-cyclic damage and finally fatigue loading - the model accounts for all the cracked phases in a unified manner



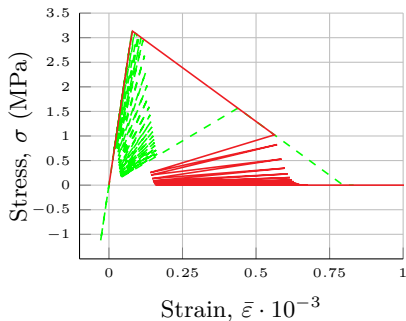
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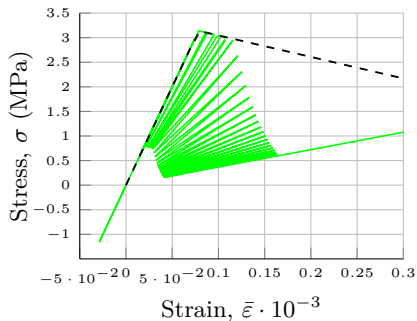
Cyclic response of fiber-hinge

- Considering a hinge subjected to constant curvature loading
- Visualising the response of a fiber on the upper part of the hinge (green) and the bottom fiber (red)
- **The bottom fiber enters fatigue after the first load step, as the lower part of the hinge deteriorate, new fibers in the upper part are activated**
- The upper fiber is first in compression, then in linear elastic tension, before entering a short stage of low-cyclic damage and finally fatigue loading - the model accounts for all the cracked phases in a unified manner

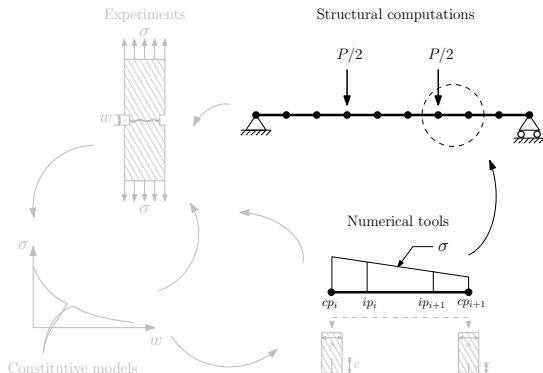


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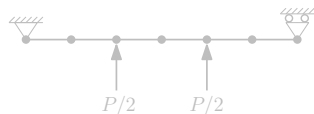
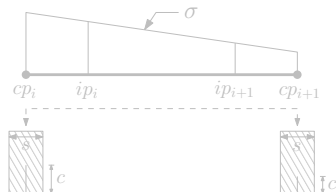
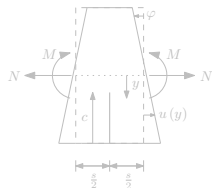


(iii) Beam level: Hinge as constitutive model in a non-linear beam element



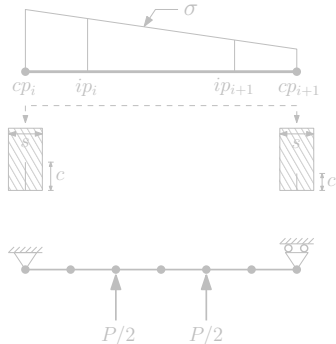
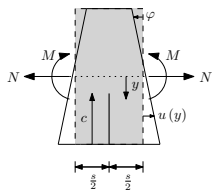
Beam model

- **The proposed hinge is implemented in a user built finite element code applying a three node beam element**
- The underlying description of the hinge is based on the formation of discrete cracks
- However, when implemented into a beam element, the constitutive behaviour of the hinge is smeared (smooth)
- Making the model applicable for structural analysis, e.g. area outside the loaded points (where the moment distribution is no longer constant)



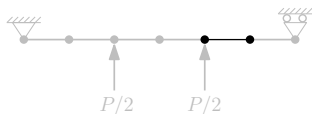
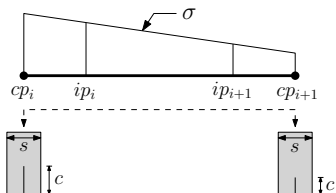
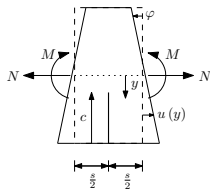
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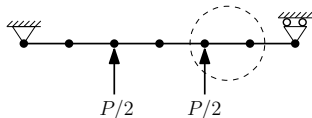
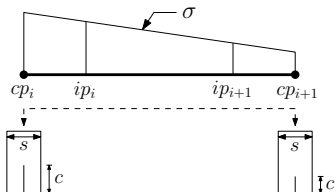
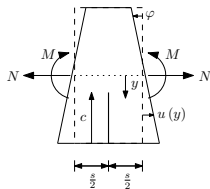
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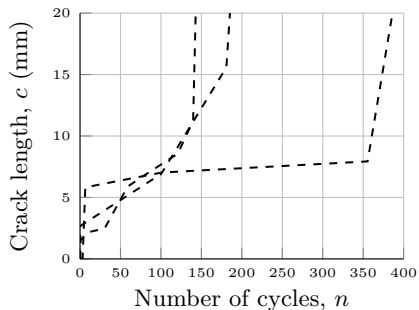


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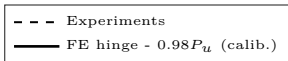
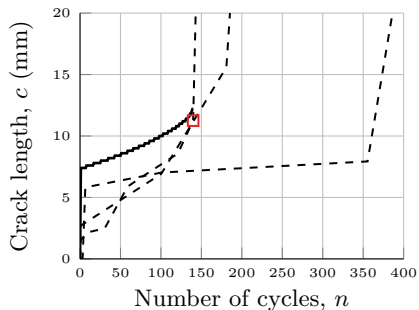
Numerical simulation of three point bending beams in fatigue (Toumi and Bascoul, 2002)



--- Experiments

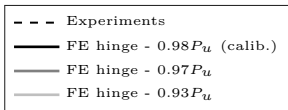
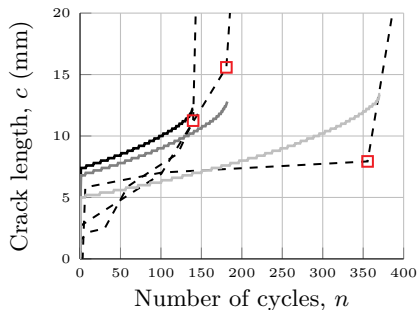
- Damage parameter k_{fat} calibrated to match the first experiment and then kept constant
- The model is able to capture the crack growth development and number of cycles to failure
- The initial crack length increase with increasing fatigue load
- The crack growth rate increase with increasing fatigue load
- The fatigue life increase for decreasing fatigue load

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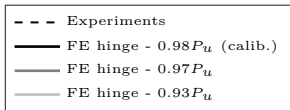
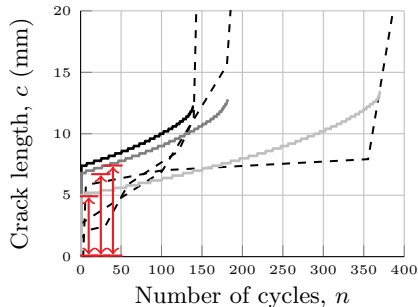
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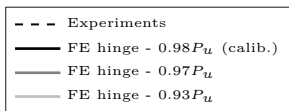
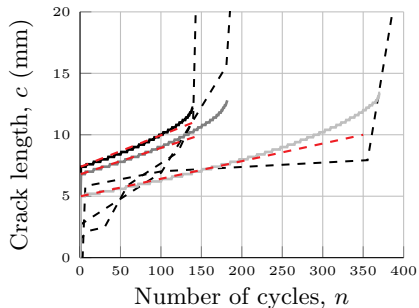
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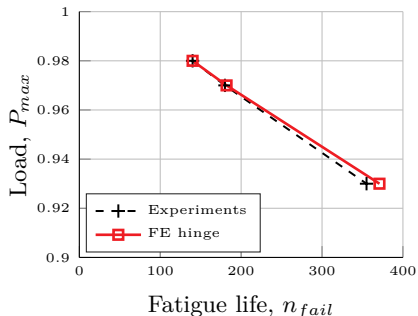
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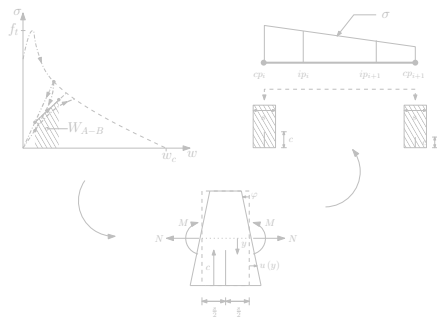
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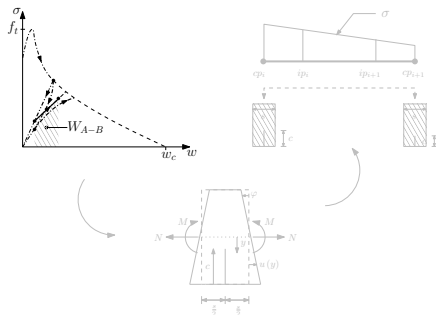
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Conclusions



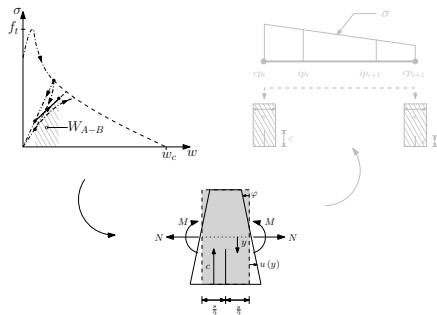
- (i) A general cyclic cohesive model was developed based on energy considerations
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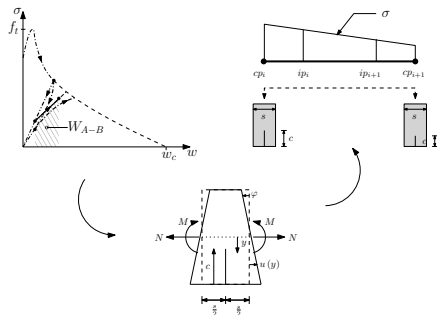
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- (i) **The selected format is general and consistent and accounts for the material behaviour in all the cracked phases**
- (ii) The proposed model shows satisfactory results when compared to experiments with cracked plain concrete cylinders in uni-axial tension
- (iii) However, experimental validation of the fatigue damage parameter k_{fat} is difficult due to lack of sufficient data
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